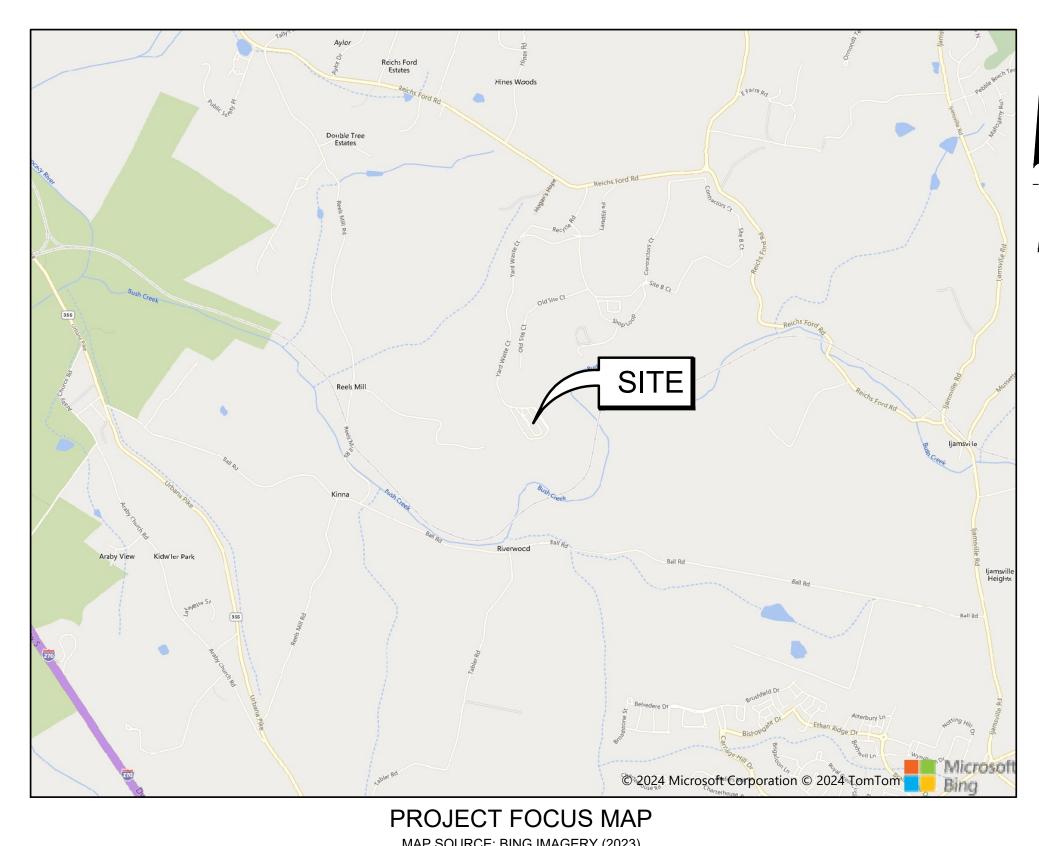


FREDERICK COUNTY TRANSFER STATION AND PROCESSING FACILITY RETROFIT DESIGN 9031 REICHS FORD ROAD, FREDERICK, MARYLAND

MARCH 2025



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PROJECT FOCUS MAP
MAP SOURCE: BING IMAGERY (2023)

0 2000 4000

PREPARED FOR:

FREDERICK COUNTY DIVISION OF SOLID WASTE AND RECYCLING 9031 REICHS FORD ROAD, FREDERICK, MARYLAND 21074

PREPARED BY:

Geosyntec

consultants

10211 WINCOPIN CIRCLE, FLOOR 4 COLUMBIA, MARYLAND 21044 PHONE: (410) 381-4333 COVER SHEET

PROJECT:

TRANSFER STATION RETROFIT

SITE:

FREDERICK COUNTY TRANSFER STATION
FREDERICK, MARYLAND

DESIGN BY:

JFB

DATE:

MARCH 2025

DAWN BY:

TCR

PROJECT TO PROJECT NO:

ME2602

CHECKED BY:

MILV

DRAWING NO:

APPROVED BY:

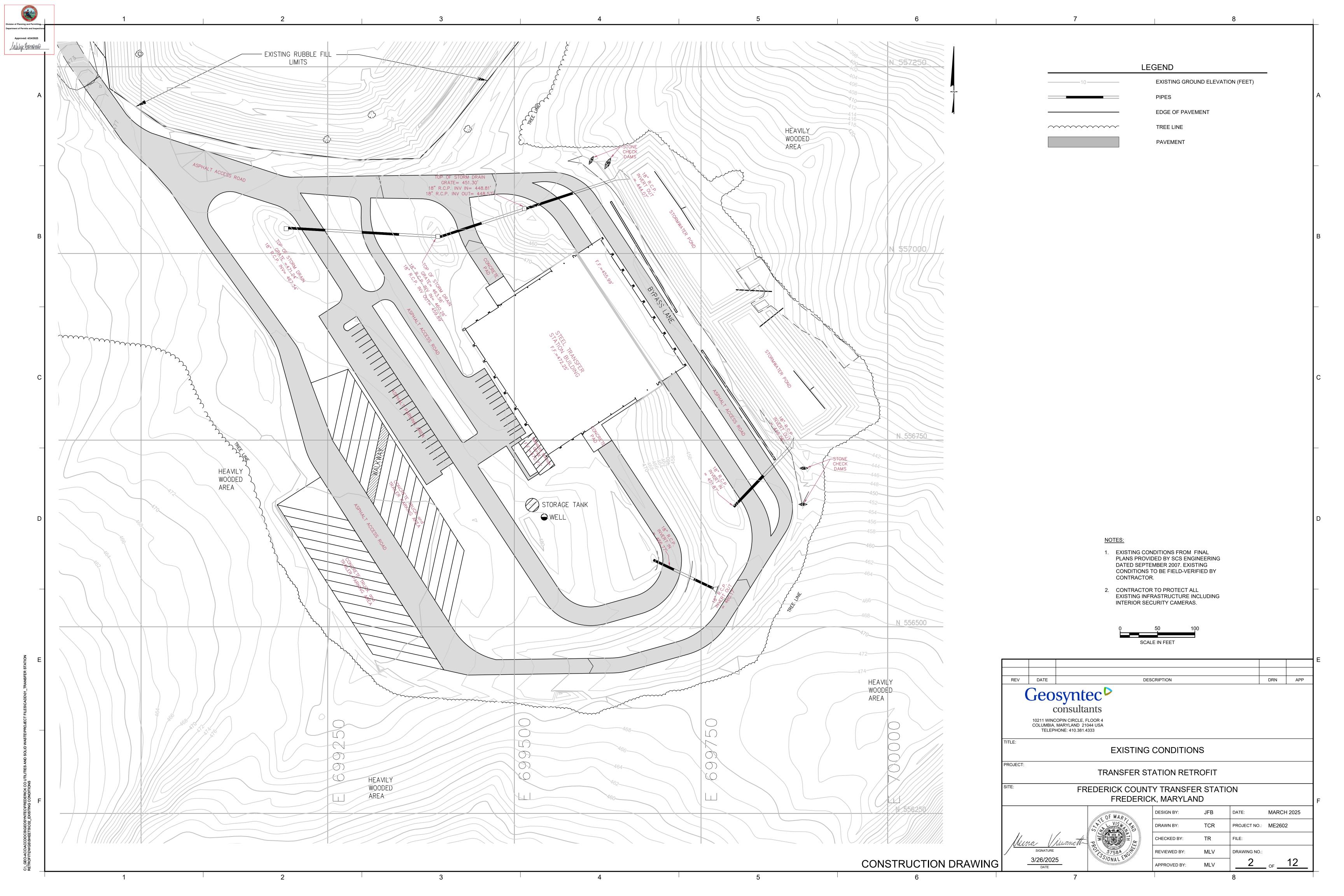
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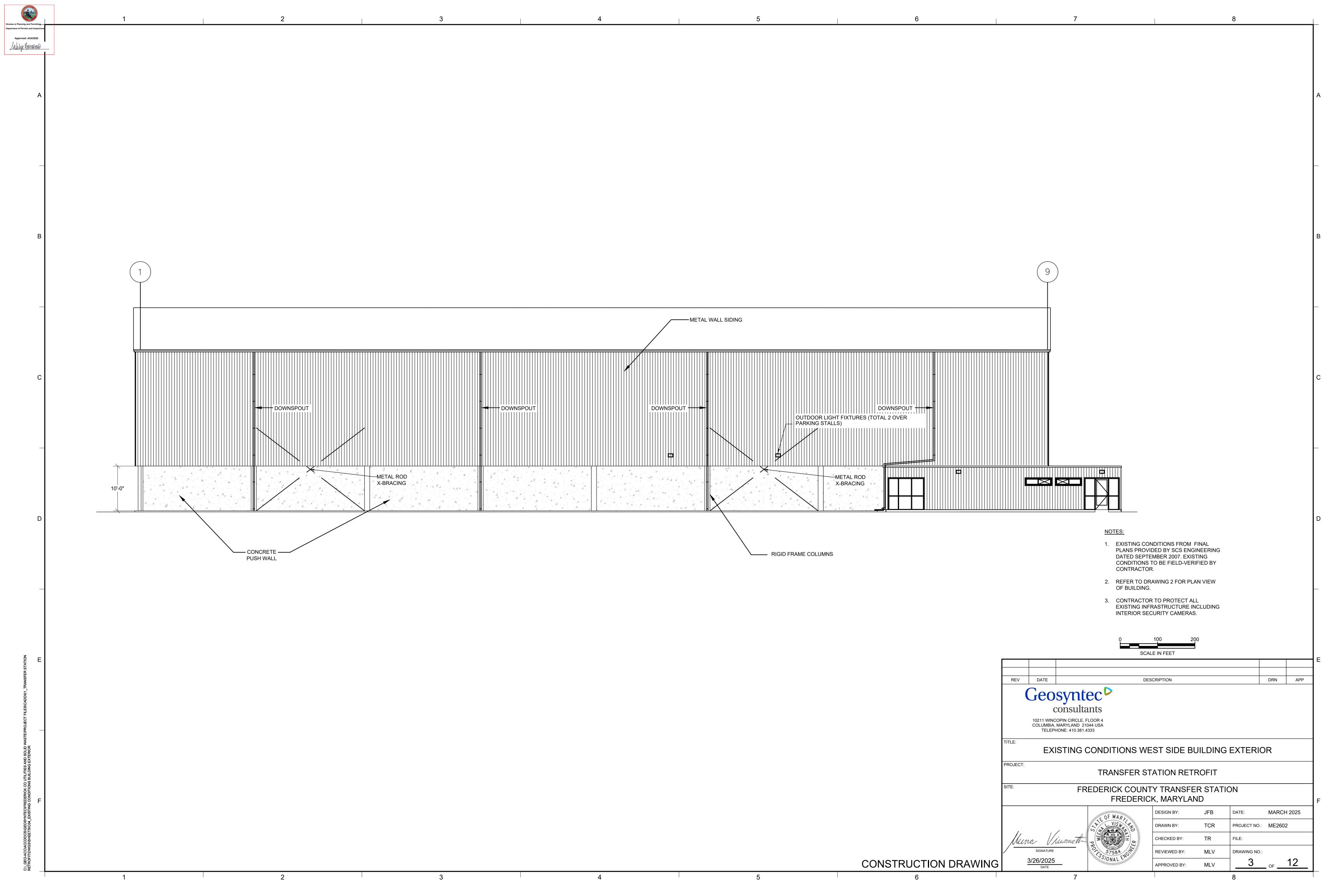
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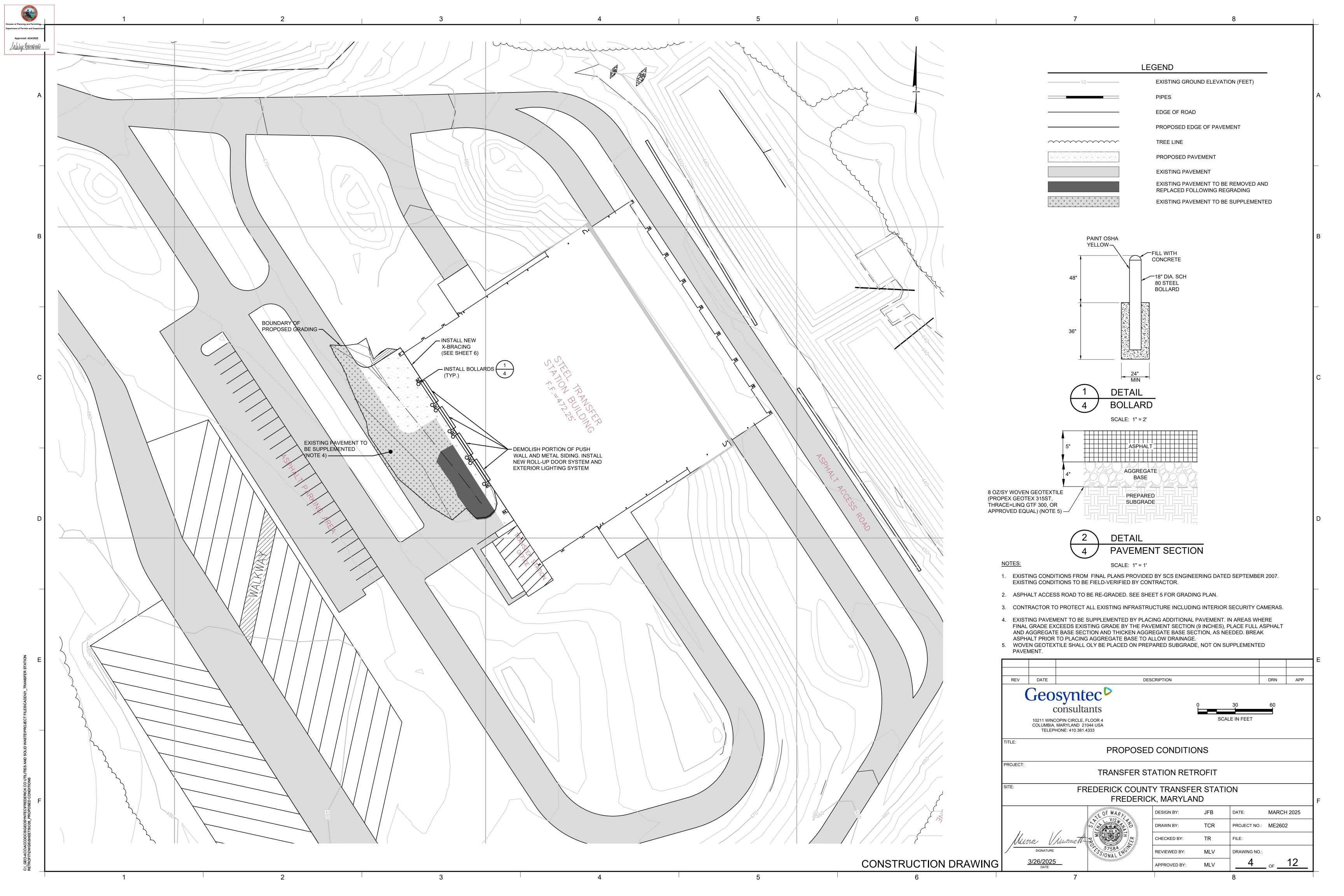
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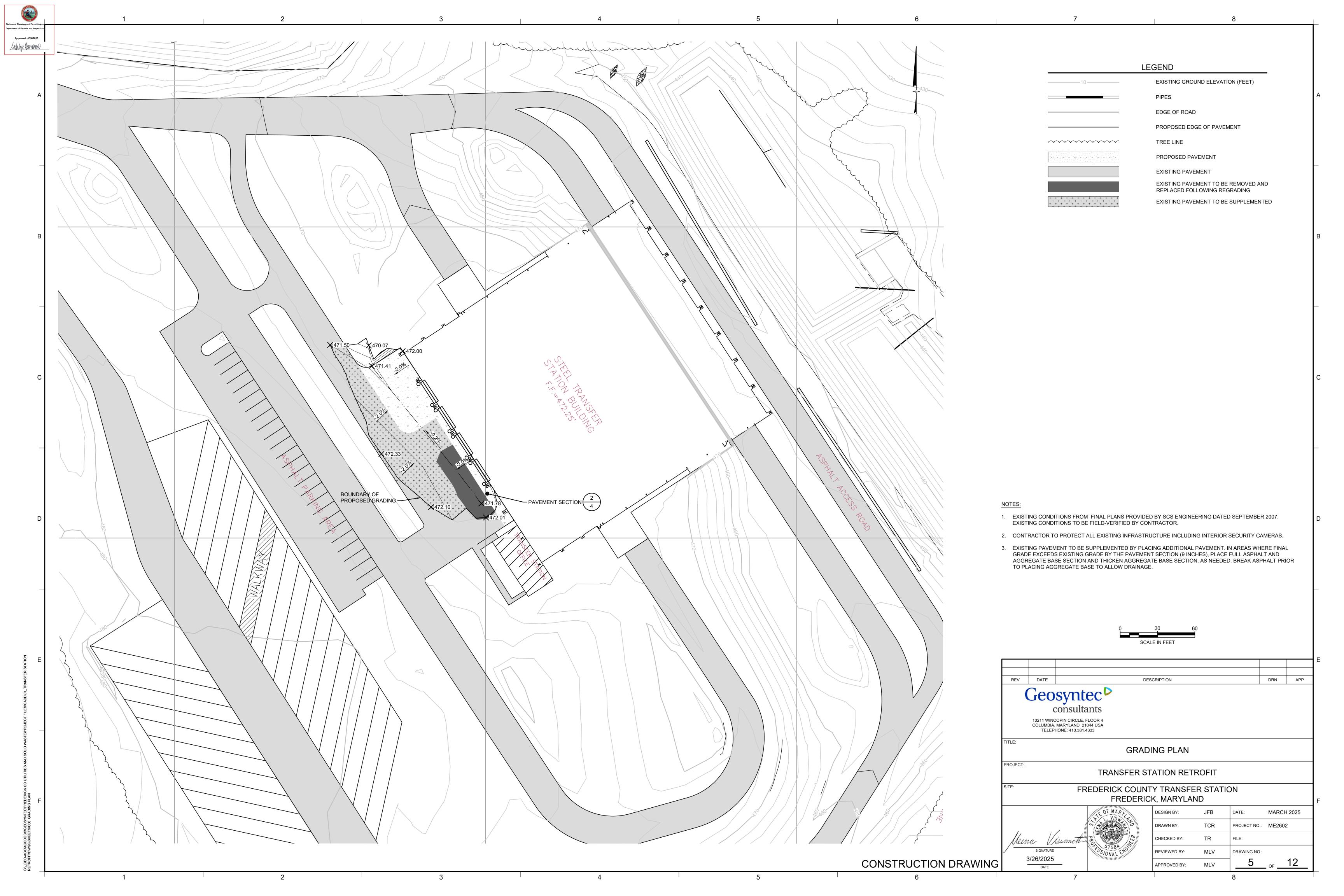
CONSTRUCTION DRAWING

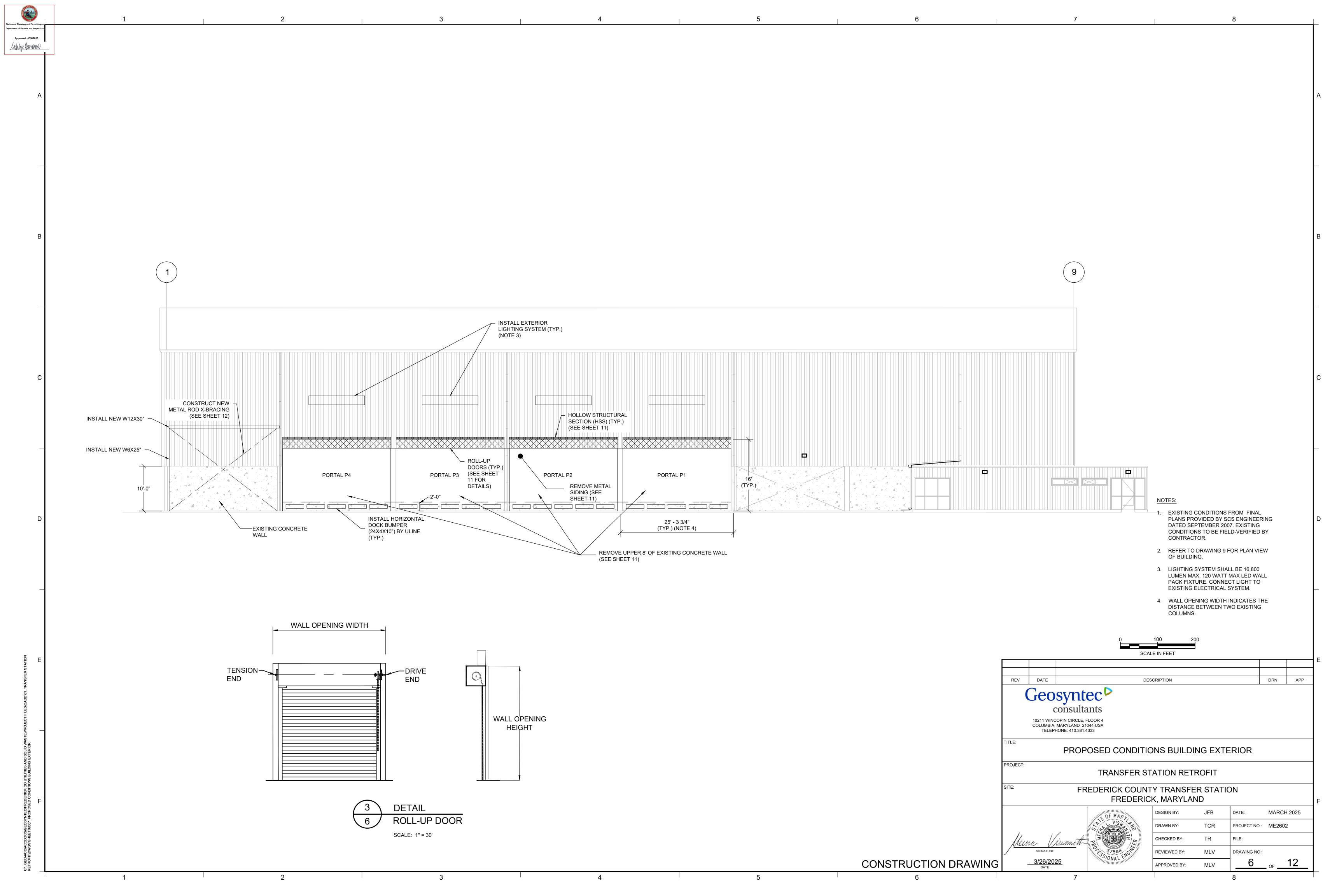
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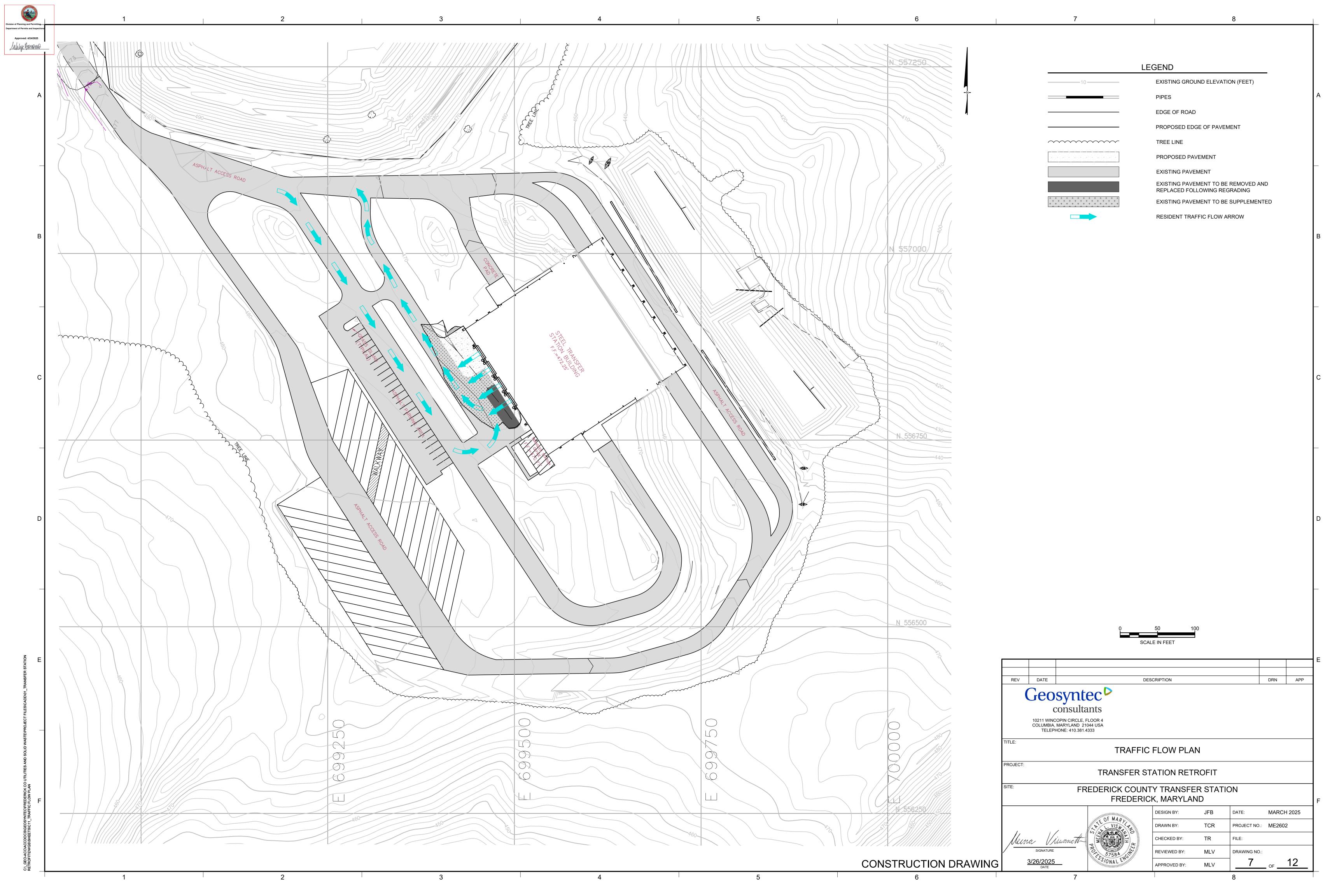












ANY REFERENCES TO VARIOUS TRADE CODES THROUGHOUT THESE NOTES ARE TO THE YEAR OF THE CODE CITED IN THE ABOVE REFERENCE BUILDING

DESIGN LIVE LOADS

ROOF STRUCTURAL ELEMENTS SHALL BE DESIGNED FOR THE MORE CRITICAL OF THE FOLLOWING LOAD CASES CASE 1 30 PSF MINIMUM (NOT REDUCIBLE) CASE 2 SNOW LOAD BASED ON 30 PSF GROUND SNOW LOAD WITH APPLICABLE DRIFT AND SLIDING LOADS

ROOF SNOW LOAD DESIGN DATA:

FLAT ROOF SNOW LOAD (Pf) - 21 PSF MINIMUM SNOW LOAD PER ASCE 7-16 -7.3.4 (Pm) - 20 PSF SNOW EXPOSURE FACTOR (Ce) – 1.0 SNOW LOAD IMPORTANCE FACTOR (I) - 1.0 THERMAL FACTOR (Ct) – 1.0

ROOF RAIN LOAD DATA:

RAIN INTENSITY (i) – 2.97 IN/HR RAIN LOAD <21 PSF ROOF DRAINAGE BY EXISTING SLOPED ROOF.

THE FLOOR AREAS HAVE BEEN DESIGNED FOR THE FOLLOWING MINIMUM LIVE LOADS. LIVE LOAD REDUCTION HAS BEEN CONSIDERED IN FLOOR AND COLUMN DESIGN.

TIPPING FLOOR 250 PSF OR HS20-44 OFFICES 50 PSF + 15 PSF PARTITION LOAD

EQUIPMENT SUPPORTS DESIGN DEAD LOADS:

30 PSF (TOTAL INCLUDING SELF WEIGHT)

150 PSF (TOTAL) FLOOR FLOOR 112 PSF (SLAB) FLOORING 38 PSF (TOPPING)

LATERAL LOADS:

WIND LOAD ANALYSIS RISK CATEGORY 115 MPH ULTIMATE WIND SPEED (Vult) 89 MPH NOMINAL WIND SPEED (Vasd) WIND EXPOSURE

INTERNAL PRESSURE COEFFICIENT +/- 0.18

NET WIND UPLIFT ON ROOF WITH RESPECT TO METAL DECK AND STEEL FRAMING TO BE PER THE LOADS IN THE COMPONENTS AND CLADDING WIND CHART ON

DESIGNED FOR ACTUAL LOADS INDICATED ON THE DRAWINGS

SEISMIC LOAD ANALYSIS RISK CATEGORY SEISMIC IMPORTANCE FACTOR(Ie) MCE SPECTRAL RESPONSE ACCELERATION PARAMETER - SHORT (Ss) 13.3%g MCE SPECTRAL RESPONSE ACCELERATION PARAMETER - 1 second (S1) 4.3%g SITE CLASS SOIL SITE COEFFICIENT (FA / FV) DESIGN EQ SPECTRAL RESPONSE ACCEL, PARAMETER - SHORT (Sds) 0.142 DESIGN EQ SPECTRAL RESPONSE ACCEL, PARAMETER - 1 second (Sd1) 0.069 SEISMIC DESIGN CATEGORY BASIC SEISMIC FORCE RESISTING SYSTEM H per TABLE 12.2-1 ASCE7-10 RESPONSE MODIFICATION COEFFICIENT (R) SYSTEM OVERSTRENGTH FACTOR (Ωo) 3.0 DEFLECTION AMPLIFICATION FACTOR (Cd) 3.0 SEISMIC RESPONSE COEFFICIENT (Cs) 0.0473 BASE SHEAR (Fx) MASS x Cs ANALYSIS PROCEDURE UTILIZED **EQUIVALENT LATERAL FORCE**

LATERAL ANALYSIS OF RENOVATION SHOWS THAT LATERAL MEMBER DEMAND-TO-CAPACITY RATIOS ARE INCREASED BY LESS THAN 10 PERCENT AFTER ALTERATIONS; NO REMEDIATION OF THE LATERAL SYSTEM IS REQUIRED PER IEBC SECTION 503.4 EXCEPTION

GENERAL NOTES

REFER TO THE ARCHITECTURAL, ELECTRICAL, MECHANICAL AND PLUMBING DRAWINGS FOR ADDITIONAL SLEEVES, ANCHORS, VENT OPENINGS, ETC. NOT

NOTIFY THE STRUCTURAL ENGINEER OF RECORD OF ANY DEVIATION FROM THE STRUCTURAL CONTRACT DOCUMENTS FOR APPROVAL (I.E. OPENINGS IN STRUCTURAL ELEMENTS SUCH AS LOAD-BEARING WALLS).

ALL MATERIALS SHALL BE IN CONFORMANCE WITH THE LATEST EDITION OF THE ASTM SPECIFICATIONS NOTED IN THE STRUCTURAL NOTES AND PROJECT SPECIFICATIONS BASED ON THE FINAL DATE NOTED ON THE CONSTRUCTION DOCUMENTS.

THIS PROJECT HAS BEEN DESIGNED FOR THE WEIGHTS OF THE MATERIALS INDICATED ON THE DRAWINGS AND FOR THE LIVE LOADS INDICATED IN THE DESIGN DATA ABOVE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE ADDITIONAL SHORING AND BRACING FOR THE STRUCTURE IF ACTUAL CONSTRUCTION LOADS EXCEED THE DESIGN LOADS.

ALL DIMENSIONS AND NOTES SHALL SUPERSEDE ALL SCALE REFERENCES ON THE DRAWINGS.

ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS, THE BUILDING CODE AND ALL LOCAL ORDINANCES. THE CONTRACTOR SHALL HIRE AN EXPERIENCED, QUALIFIED SPECIAL INSPECTOR TO PERFORM ALL THE REQUIRED INSPECTION WORK. ADTEK ENGINEERS WILL NOT PERFORM THE CONTINUOUS DAILY SPECIAL INSPECTIONS DURING CONSTRUCTION. ADTEK ENGINEERS MAY VISIT THE SITE TO ASCERTAIN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS AND SUCH VISITS ARE NOT TO BE CONSTRUED AS MEETING THE DAILY SPECIAL INSPECTION REQUIREMENTS UNLESS THE ENGINEER SPECIFICALLY SO STATES IN WRITING.

IT IS THE INTENT OF THESE DRAWINGS FOR ALL DISCIPLINES AND SPECIFICATIONS TO PRODUCE A COMPLETE PROJECT. IN ALL CASES THE DRAWINGS AND SPECIFICATIONS MUST BE REVIEWED, PRICED, ESTIMATED, AND CONSTRUCTED IN THEIR ENTIRETY. THE DRAWINGS ARE COMPLEMENTARY TO ONE ANOTHER AND THE SPECIFICATIONS. ANYTHING SHOWN OR IMPLIED ON ANY ONE DRAWING MUST BE PROVIDED, INSTALLED AND CONNECTED AS THOUGH IT WAS SHOWN ON ALL DRAWINGS AND INCLUDED IN THE ORIGINAL PRICING. NO REQUEST FOR ADDITIONAL COST OR CHANGE ORDER WILL BE ACCEPTED BY THE OWNER FROM ANY CONTRACTOR, SUPPLIER, OR INSTALLER THAT RESULTS FROM A FAILURE TO THOROUGHLY REVIEW ALL DRAWINGS AND SPECIFICATIONS, COORDINATE WITH OTHER TRADES, OR THOROUGHLY INSPECT THE SITE TO DETERMINE ALL EXISTING CONDITIONS

IF AN ASSUMED OR ACTUAL CONFLICT IS DISCOVERED IN THE CONTRACT DOCUMENTS, THE MORE EXPENSIVE OR HIGHER QUALITY OPTION (AS DETERMINED BY THE ARCHITECT/ENGINEER) SHALL BE ASSUMED TO APPLY UNLESS DIRECTED OTHERWISE BY THE ARCHITECT/ENGINEER

THE CONTRACTOR IS REQUIRED TO VISIT THE SITE, FAMILIARIZE THEMSELVES WITH THE LOCAL CONDITIONS UNDER WHICH THE WORK IS TO BE PERFORMED AND AS ARE NECESSARY FOR CONSTRUCTION. AND CORRELATE THEIR OBSERVATIONS WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. IT IS ASSUMED THAT THE CONTRACTOR HAS OBTAINED, BEFORE AWARD OF THE CONTRACT, CLARIFICATION OF ALL QUESTIONS AS TO THE INTENT OF THE CONTRACT DOCUMENTS AND OF ASSUMED OR ACTUAL CONFLICT BETWEEN TWO OR MORE ITEMS IN CONTRACT DOCUMENTS. SHOULD THE CONTRACTOR FAIL TO OBTAIN SUCH CLARIFICATION, THE ARCHITECT/ENGINEER SHALL DIRECT WORK TO PROCEED BY THE METHOD INDICATED, SPECIFIED OR REQUIRED BY CONTRACT DOCUMENTS WHICH WILL PRODUCE THE BEST RESULTS, AS JUDGED BY THE ARCHITECT/ENGINEER. SUCH DIRECTION BY THE ARCHITECT/ENGINEER SHALL NOT ENTITLE THE CONTRACTOR TO ANY CLAIM FOR EXTRA COST

CONTRACTOR RESPONSIBILITIES

THE FOLLOWING LIST IS NOT INTENDED TO BE ALL INCLUSIVE, BUT MERELY TO PLACE EMPHASIS ON PARTICULAR ITEMS OF JOB SCHEDULING AND SAFETY.

- 1. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS TO THE PROJECT DESIGN TEAM FOR REVIEW. ALLOWING A MINIMUM OF TWO WEEKS FOR REVIEW BY THE PROJECT ARCHITECT AND STRUCTURAL ENGINEER.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REDESIGN OF THE STRUCTURAL SUPPORTS OF EQUIPMENT WHEN THE OPERATING WEIGHT OF THE EQUIPMENT PROVIDED (INCLUDING CURBS AND ACCESSORIES) EXCEEDS THE MAXIMUM DESIGN WEIGHTS NOTED ON THE STRUCTURAL DRAWINGS. SUBMIT STRUCTURAL CALCULATIONS AND DETAILS FOR THE REVISED EQUIPMENT SUPPORT TO THE PROJECT ARCHITECT FOR REVIEW. THE SUBMITTAL SHALL BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT JURISDICTION.
- 3. IF ACTUAL FIELD CONDITIONS VARY FROM WHAT IS SHOWN OR ASSUMED IN THE CONTRACT DOCUMENTS. THE CONTACTOR IS REQUIRED TO PROMPTLY NOTIFY THE ARCHITECT/ENGINEER AND RECEIVE DIRECTION PRIOR TO PROCEEDING WITH THE WORK AFFECTED BY THE ACTUAL FIELD CONDITION.
- 4. THE CONTRACTOR SHALL NOTIFY THE PROJECT SPECIAL INSPECTOR IN ADVANCE OF WORK REQUIRING INSPECTIONS OR ON-SITE PERSONNEL. COORDINATE ADVANCE NOTIFICATION REQUIREMENTS WITH THE SPECIAL INSPECTOR.
- 5. IF THE CONTRACTOR ANTICIPATES A PROBLEM THAT WILL REQUIRE ASSISTANCE FROM THE PROJECT STRUCTURAL ENGINEER, THE CONTRACTOR SHALL MAKE EVERY EFFORT TO PROVIDE THE ENGINEER WITH MINIMUM 24 HOURS NOTICE.
- 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ENSURING THAT ALL CONSTRUCTION IS ACCORDING TO THE SIGNED AND SEALED CONSTRUCTION DOCUMENTS AND THE REVIEWED SHOP DRAWINGS.
- 7. THE CONTRACTOR SHALL ENGAGE A PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT JURISDICTION TO DESIGN AND DETAIL THE SUBMITTAL ITEMS NOTED IN THE DEFERRED SUBMITTALS BELOW.
- 8. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING BETWEEN THE STRUCTURAL AND ARCHITECTURAL DRAWINGS. IT IS NOT INTENDED THAT THE STRUCTURAL DRAWINGS BE USED INDEPENDENTLY OF THE ARCHITECTURAL DRAWINGS. ANY DISCREPANCIES, INCLUDING DIMENSIONS, SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH THE WORK.
- 9. THE CONTRACTOR IS RESPONSIBLE FOR METHODS TO ENSURE CONSTRUCTION SAFETY AT THE SITE THROUGHOUT THE COURSE OF THE PROJECT CONSTRUCTION. SEE O.S.H.A. & M.O.S.H. REGULATIONS FOR CONSTRUCTION.
- 10. UPON STRUCTURAL COMPLETION OF THE PROJECT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING THE SPECIAL INSPECTOR SUBMIT A LETTER OF CERTIFICATION INDICATING THAT THE STRUCTURE IS IN COMPLIANCE WITH THE PLANS, SPECIFICATIONS, CONCRETE TEST REPORTS AND CODE REQUIREMENTS. THIS LETTER MUST BE REVIEWED BY THE ARCHITECT AND ENGINEER OF RECORD BEFORE SUBMITTAL

SUBMITTALS NOTES

1. SUBMIT THE SHOP DRAWINGS NOTED BELOW TO THE DESIGN TEAM FOR REVIEW.

- 2. REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONSTRUCTION DOCUMENTS FOR USE AS SHOP DRAWINGS IS PROHIBITED.

 - A. CONCRETE MIX DESIGN BY EITHER TRIAL BATCH OR FIELD EXPERIENCE METHODS. (EACH SUBMITTED MIX MUST IDENTIFY ITS INTENDED USE)
- B. CONCRETE REINFORCING C. STRUCTURAL STEEL
- D. ANY OPENINGS IN NEW AND EXISTING STRUCTURAL ELEMENTS NEED TO BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.

SPECIAL INSPECTIONS

SPECIAL INSPECTIONS ARE REQUIRED DURING CONSTRUCTION IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE. THE TERM SPECIAL INSPECTOR REFERS TO THE SPECIAL INSPECTING ENGINEER OF RECORD HIRED BY THE OWNER IN COMPLIANCE WITH THE INTERNATIONAL BUILDING CODE. INSPECTIONS OF FOUNDATION SUBGRADES MUST BE CONDUCTED BY A LICENSED GEOTECHNICAL ENGINEER, REFERRED TO HEREIN AS THE GEOTECHNICAL INSPECTOR. SPECIAL INSPECTIONS SHALL BE PERFORMED FOR, BUT NOT LIMITED TO, THE FOLLOWING STRUCTURAL ITEMS:

- 1. FOUNDATION REINFORCING
- 2. CONCRETE FORMWORK AND REINFORCING
- 3. CONCRETE MIX AND PLACEMENT
- 4. STRUCTURAL STEEL ERECTION POST-INSTALLED ANCHORS
- **FOUNDATION**

ASSUMED SOIL BEARING VALUE:

2.500 POUNDS PER SQUARE FOOT FOR COLUMN AND WALL FOOTINGS.

EXISTING CONDITIONS

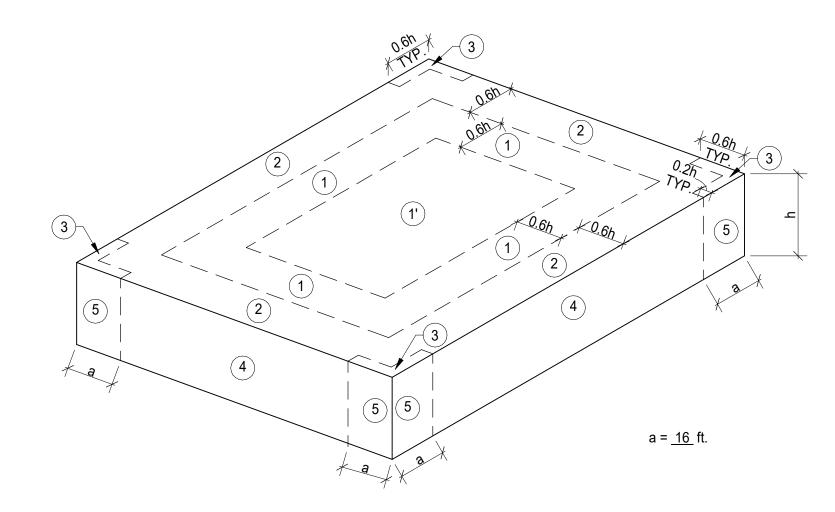
ALL EXISTING CONDITIONS SHALL BE CHECKED AND VERIFIED IN THE FIELD BEFORE EXCAVATION, DEMOLITION, OR CONSTRUCTION IS BEGUN. EXISTING UTILITIES SHALL BE LOCATED AND PROTECTED AS REQUIRED BY THE EXCAVATION, DEMOLITION, OR CONSTRUCTION. FIELD MEASUREMENTS SHALL BE MADE OF ADJOINING CONSTRUCTION RELATIVE TO THE PROPER INSTALLATION OF NEW WORK. ALL DISCREPANCIES SHALL BE REPORTED TO THE PROJECT ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH THE WORK IN THE AREA OF THE DISCREPANCY.

FIELD VERIFY ALL RELEVANT EXISTING DIMENSIONS, ELEVATIONS, AND MEMBER SIZES.

SHORING OF BUILDING STRUCTURAL MEMBERS IF REQUIRED DURING DEMOLISH OF PUSH WALL

SHORING OF STRUCTURAL ELEMENTS SHALL BE PROVIDED AS REQUIRED TO PROTECT EXISTING CONSTRUCTION. THE SHORING DESIGN SHALL BE AS REQUIRED BY THE CONSTRUCTION. SUBMIT SHOP DRAWINGS SHOWING ACTUAL SEQUENCE AND DETAILS OF THE SHORING PROCEDURE, AS WELL AS CALCULATIONS INDICATING THAT THE EXISTING BUILDING LOADS AND CONSTRUCTION LOADS HAVE BEEN ACCOMMODATED IN THE SHORING DESIGN. SEE THE "CONTRACTOR RESPONSIBILITIES" AND "SUBMITTAL" NOTES ABOVE FOR ADDITIONAL REQUIREMENTS.

THE CONTRACTOR SHALL VERIFY THE CONDITION OF THE EXISTING STRUCTURE IN THE AREA OF THE PROPOSED SHORING. THE CONTRACTOR IS RESPONSIBLE FOR THE COST OF THE DESIGN AND REPAIR OF EXISTING STRUCTURES AND/OR FINISHES DAMAGED DURING SHORING OPERATIONS.



	WIND COMPONENT & CLADDING LOAD SCHEDULE						ULE
·	POSITIVE PRESSURE		NEGATIVE PRESSURE				
AREA (SQ. FT.)	ZONE 4 (PSF)	ZONE 5 (PSF)	ZONE 1 (PSF)	ZONE 2	ZONE 3	ZONE 4	ZONE 5
10	25.94	25.94	-41.31	-54.5	-74.23	-28.12	-34.77
20	24.74	24.74	-38.59	-51.01	-67.25	-26.92	-32.37
50	23.22	23.22	-34.99	-46.33	-57.99	-25.40	-29.31
100	22.02	22.02	-32.26	-42.84	-50.01	-24.20	-26.92

POSITIVE AND NEGATIVE SIGNS ON VALUES IN SCHEDULE INDICATE PRESSURES ACTING

- TOWARD AND AWAY FROM THE SURFACE, RESPECTIVELY. ZONES 1, 2, AND 3 ARE FOR ROOFS. ZONES 4 AND 5 ARE FOR WALLS. SEE DIAGRAM ABOVE FOR IDENTIFYING EXTENTS OF WIND PRESSURE ZONES.
- VALUES GIVEN ARE FOR 700 YEAR MRI (MEAN RECURRENCE INTERVAL) AS REQUIRED FOR STRENGTH CALCULATIONS. ALTERNATE DESIGN VALUES MAY BE USED PROVIDED SUPPORTING CALCULATIONS. SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE PROJECT'S JURISDICTION, ARE SUBMITTED FOR REVIEW.

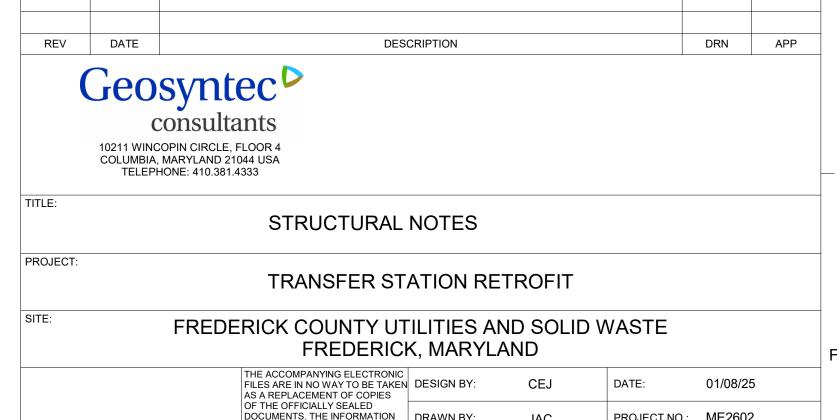




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CONSTRUCTION DRAWINGS



PROJECT NO.: ME2602 IS SUPPLIED "AS IS" AND ANY MAKING USE OF OR RELYING UPON CHECKED BY: FILE: THIS DATA IS RESPONSIBLE FOR ONFIRMING ITS ACCURACY AN COMPLETENESS THESE FILES

DRAWING NO.: ARE NOT STAMPED OR SEALED AND ONLY DRAWINGS WITH APPROPRIATE STAMP OR SEAL ARE TO BE CONSIDERED AS FINAL. APPROVED BY:

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ND SEALED DOCUMENTS



Approved: 4/24/2025

Salye Gonomo

STRUCTURAL CONCRETE

REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. ALL WELDED WIRE REINFORCING SHALL CONFORM TO ASTM A1064. DETAILING SHALL BE IN _ACCORDANCE WITH ACI MANUAL 315 AND STANDARD 318.

CONCRETE SHALL BE NORMAL WEIGHT. WITH A 28 DAY COMPRESSIVE STRENGTH = 4,500 PSI WITH 0.45 WATER/CEMENT RATIO AND 6% ± 1.5 AIR ENTRAINMENT.

MAXIMUM AGGREGATE SIZE FOR CONCRETE SHALL BE IN ACCORDANCE WITH THE MAXIMUM AGGREGATE SIZES IN ACI 318 AND AS FOLLOWS:

CONCRETE PIERS, WALLS CONCRETE SLABS ON GRADE 3/4" 3/4"

ALL EXTERIOR CONCRETE AND CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED.

CONCRETE SLUMP: 3" +/- 1"

8" AFTER ADDITION OF HRWR AT THE SITE

THE USE OF ADDITIVES SHALL NOT BE PERMITTED UNLESS SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER OR NOTED IN THE PROJECT SPECIFICATIONS. THE USE OF ADDITIVES CONTAINING CALCIUM CHLORIDE SHALL NOT BE PERMITTED.

PROVIDE A HIGH RANGE WATER REDUCER (HRWR OR SUPERPLASTICIZER) FOR PUMPED CONCRETE AND AS REQUIRED FOR WORKABILITY.

ALL REINFORCING STEEL MARKED "CONTINUOUS" SHALL BE LAPPED AS REQUIRED WITH CLASS B TENSION SPLICES PER ACI 315. PROVIDE CLASS B TENSION SPLICES AT WALL CORNERS AND INTERSECTIONS WITH STANDARD 90 DEGREE BENT CORNER BARS, INCLUDING CORNERS OF WALL FOOTINGS AND BOND BEAMS. LAP WELDED WIRE REINFORCING ONE FULL SQUARE AT SIDE AND END LAPS. PROVIDE CORNER LAP BARS AT ALL LONGITUDINAL FOOTING REINFORCING AS WELL AS AT ALL HORIZONTAL

PROVIDE CONCRETE PROTECTION FOR REINFORCING AS FOLLOWS (UNLESS NOTED OTHERWISE):

PIERS: 1-1/2" TO THE TIES

ALL CONCRETE WORK, REINFORCING PLACEMENT FORMWORK SHALL BE INSPECTED UNDER THE SUPERVISION OF THE SPECIAL INSPECTOR. CONCRETE QUALITY CONTROL, INSPECTION AND TESTING SHALL BE IN STRICT ACCORDANCE WITH THE PROJECT SPECIFICATIONS, ACI 301 AND THE LOCAL BUILDING CODE REQUIREMENTS.

CONSTRUCTION PRACTICES:

FORMWORK DESIGN, SHORING, AND BRACING SHALL BE ACCORDING TO ACI 301.

FORMWORK TOLERANCES SHALL BE PER ACI 117.

THE SPECIAL INSPECTOR SHALL PERFORM A MINIMUM OF ONE CONCRETE TEST FOR EACH 50 CUBIC YARDS OF CONCRETE POURED AT THE PROJECT WITH AT LEAST ONE TEST FOR EACH DAY THAT CONCRETE IS POURED. EACH CONCRETE TEST SHALL INCLUDE A SLUMP TEST AND FIVE LABORATORY CURED TEST CYLINDERS FOR COMPRESSIVE STRENGTH TESTS. TEST TWO CYLINDERS AT 7 DAYS AFTER THE CONCRETE POUR AND TWO AT 28 DAYS WITH ONE RESERVE CYLINDER. THE SPECIAL INSPECTOR SHALL SUBMIT WRITTEN TEST REPORTS TO THE PROJECT ARCHITECT AND STRUCTURAL ENGINEER. THE ARCHITECT AND STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ALL TESTS THAT DO NOT MEET THE PROJECT SPECIFICATIONS WITHIN 24 HOURS.

STRUCTURAL STEEL

ALL STEEL SHALL BE IN ACCORDANCE WITH THE SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AISC 360, BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION

ALL STEEL W SHAPES SHALL BE ASTM A992, GRADE 50. ALL ANGLES, CHANNELS, BENT PLATES, FLAT STOCK AND OTHER MISC. METAL SHAPES SHALL BE ASTM A36 UNLESS NOTED OTHERWISE. ALL CONNECTIONS SHALL BE WELDED OR BOLTED.

HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO ASTM A500, GRADE C.

ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 36.

SHOP AND FIELD FASTENERS SHALL BE ASTM A325 HIGH STRENGTH BOLTS IN BEARING TYPE CONNECTIONS, UNLESS NOTED OTHERWISE.

NATURAL CAMBER OF STEEL BEAMS TO BE FABRICATED WITH CAMBER "UP". ANY ADDITIONAL CAMBER TO BE FABRICATED WITH CAMBER "UP". ERECTION OF ALL BEAMS TO BE CAMBER "UP".

HOLES SHALL NOT BE CUT THROUGH BEAMS AND COLUMNS UNLESS INDICATED OR APPROVED BY THE STRUCTURAL ENGINEER.

WELDING SHALL BE DONE ONLY BY AWS CERTIFIED WELDERS. WELD IN ACCORDANCE WITH THE AWS "STANDARD CODE" FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION. USE E70XX ELECTRODES.

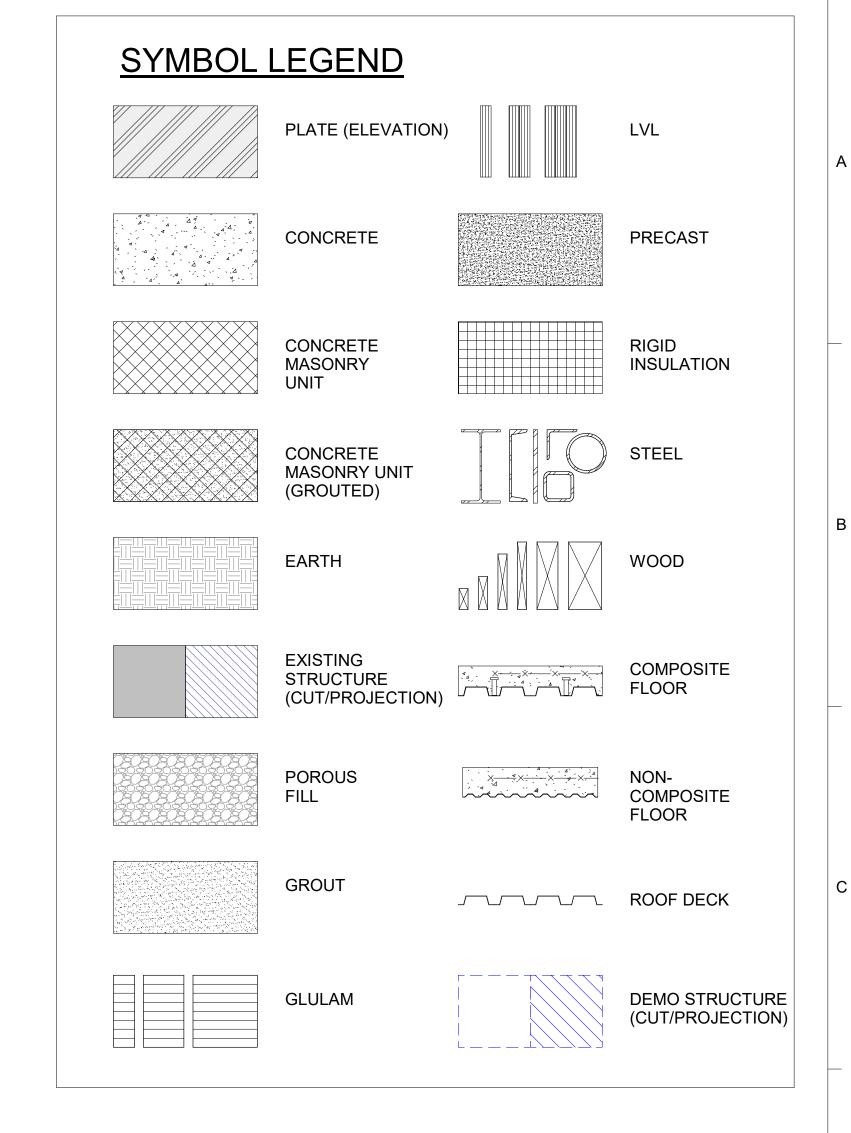
PROVIDE ADEQUATE BRACING AND GUY-WIRING FOR STEEL MEMBERS DURING STEEL ERECTION PRIOR TO FLOOR AND ROOF CONSTRUCTION. THE STEEL FRAME SHALL BE PLUMB WITHIN THE TOLERANCES IN THE AISC AND PROJECT SPECIFICATIONS. STEEL COLUMNS HAVE NOT BEEN DESIGNED AS SELF SUPPORTING, AND MUST BE ADEQUATELY BRACED DURING ERECTION.

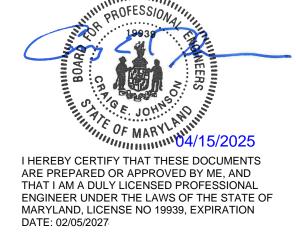
SEE THE "CONTRACTOR RESPONSIBILITES" AND "SUBMITTAL" NOTES FOR ADDITIONAL STEEL SHOP DRAWING REQUIREMENTS.

STRUCTURAL STEEL EXPOSED TO WEATHER:

ALL EXPOSED STEEL SHALL BE HOT DIPPED GALVANIZED PER ASTM A123 AFTER FABRICATION. APPLY ZINC PRIMER TO BOLTED AND WELDED CONNECTIONS IN THE FIELD.

_	RAL ABBREVIATIONS		
& @	AND AT	I IF	INSIDE FACE
	DEGREE	IN	INCH
Ø	DIAMETER	INT.	INTERIOR
= #	EQUAL NUMBER/POUNDS	J	
τ <u>t</u>	PLUS OR MINUS	J.B.E.	JOIST BEARING ELEVATION
٨		JST	JOIST
4 AB	ANCHOR BOLT	JT	JOINT
ARCH.	ARCHITECT, ARCHITECTURAL	K	
ACI	AMERICAN CONCRETE INSTITUTE	k, K	KIP
AISC	AMERICAN INSTITUTE OF		
ASCE	STEEL CONSTRUCTION AMERICAN SOCIETY OF CIVIL ENGINEERS	L	
AISI	AMERICAN IRON AND STEEL INSTITUTE	LB	POUND
ASTM	AMERICAN SOCIETY FOR	LLH LLV	LONG LEG HORIZONTAL LONG LEG VERTICAL
A14/O	TESTING AND MATERIALS	LT	LIGHT
AWS	AMERICAN WELDING SOCIETY		
В		M	MANAMA
BCX	BOTTOM CHORD EXTENSION	MAX MC	MAXIMUM MOMENT CONNECTION
BEW	BOTTOM EACH WAY	MECH	MECHANICAL
B.F. BM	BRACED FRAME BEAM	MFR	MANUFACTURER
BOTT	BOTTOM	MIN	MINIMUM
В.О.	BOTTOM OF	MO MTL	MASONRY OPENING METAL
B PL, BP BRG	BEARING PLATE / BASE PLATE	2	
BRG BTWN	BEARING BETWEEN	N	
	 .	(N)	NEW
C	CONTROL IONET / CONTROL	NO. NCMA	NUMBER NATIONAL CONCRETE AND
CJ CL	CONTROL JOINT / CONSTRUCTION JOINT CENTERLINE	IVOIVIA	MASONRY ASSOCIATION
CFS	COLD FORMED STEEL	0	
CIP	CAST IN PLACE	O.C.	ON CENTER
CLR	CLEAR	OF OH	OUTSIDE FACE OPPOSITE HAND
COL CONC	COLUMN CONCRETE	OPP	OPPOSITE
CONN	CONNECTION	OSHA	OCCUPATIONAL SAFETY
CONSTR	CONSTRUCTION		HEALTH ADMINISTRATION
CONT CRSI	CONTINUOUS CONCRETE REINFORCING STEEL INSTITUTE	P	
CVR	CONCRETE REINFORCING STEEL INSTITUTE COVER	PAF	POWDER ACTUATED FASTENER
		PL PLUMB	PLATE PLUMBING
D		PLWD	PLYWOOD
DEFL	DEFLECTION	P/C	PRECAST
DIA DN	DIAMETER DOWN	PSF PSI	POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH
DWG, DWGs	DRAWING / DRAWINGS	FOI	I GUNDO FEN OQUANE INCH
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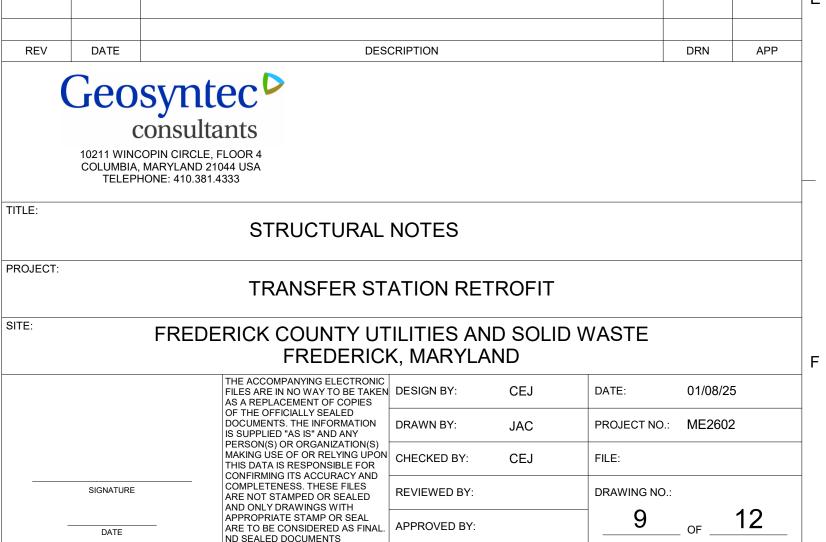




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